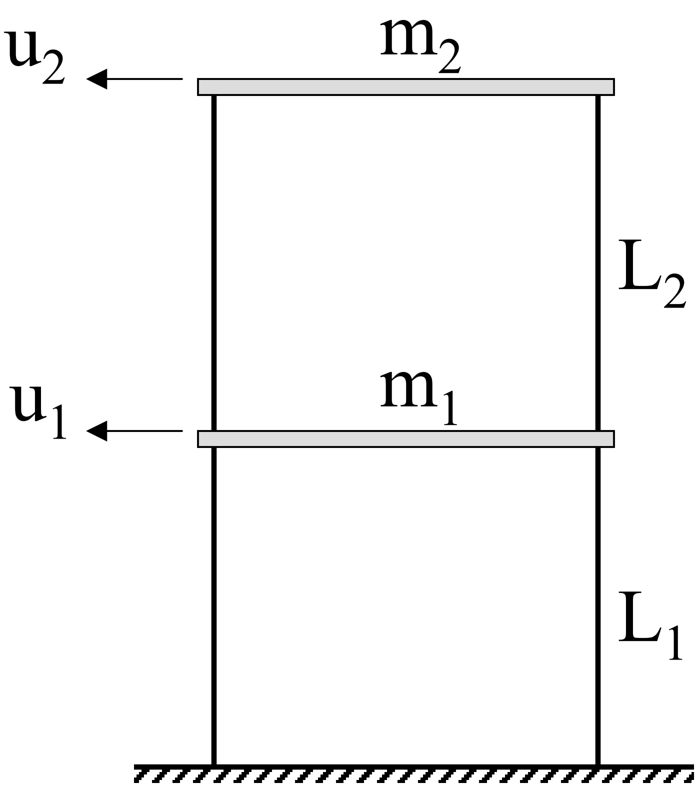
SE 180

Final Project

2 Story Shear Frame

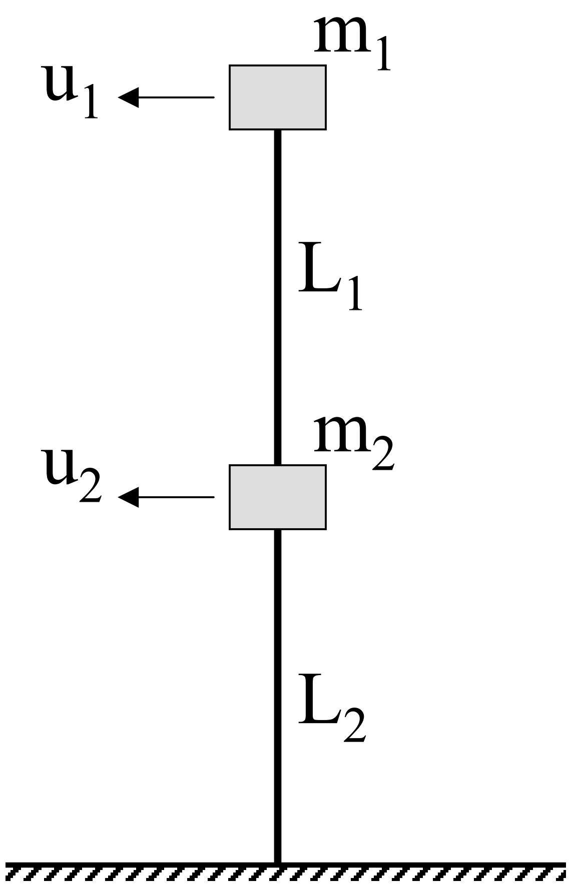


Given:

m_1
L_1
L_2
EI
ω_1
ω_2

Solve for m_2

2 Story Bending Beam



Given:

m_1
L_1
L_2
EI
ω_1
ω_2

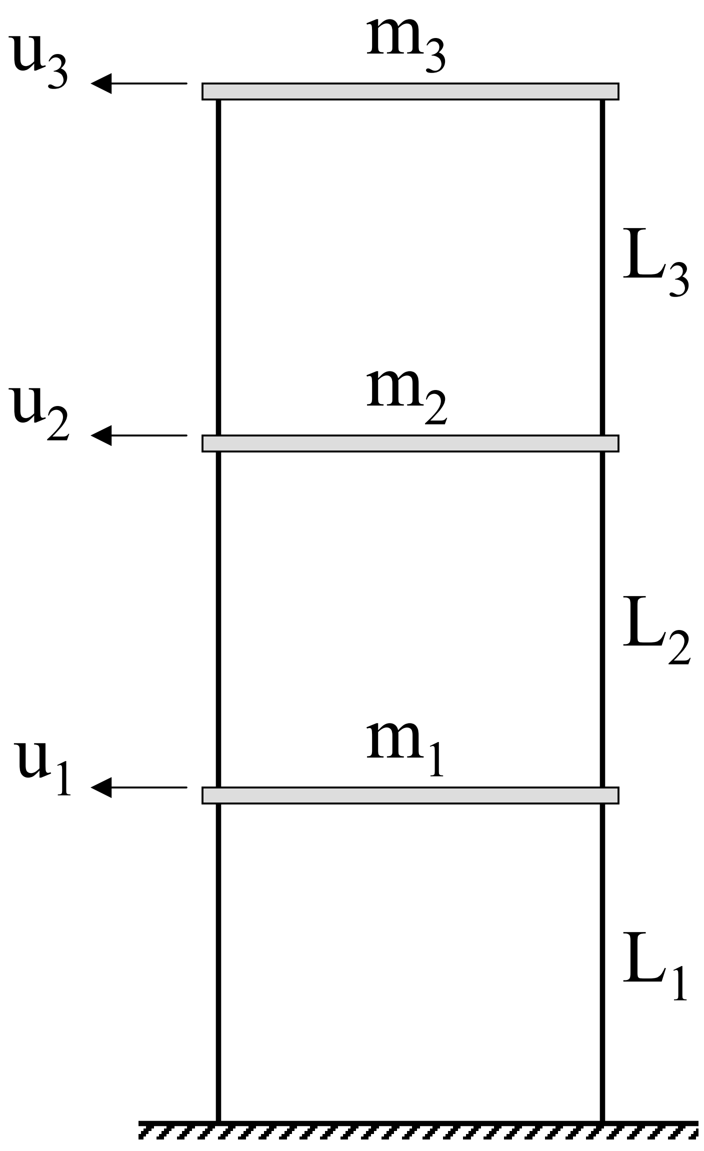
Solve for m_2

3 Story Shear Frame

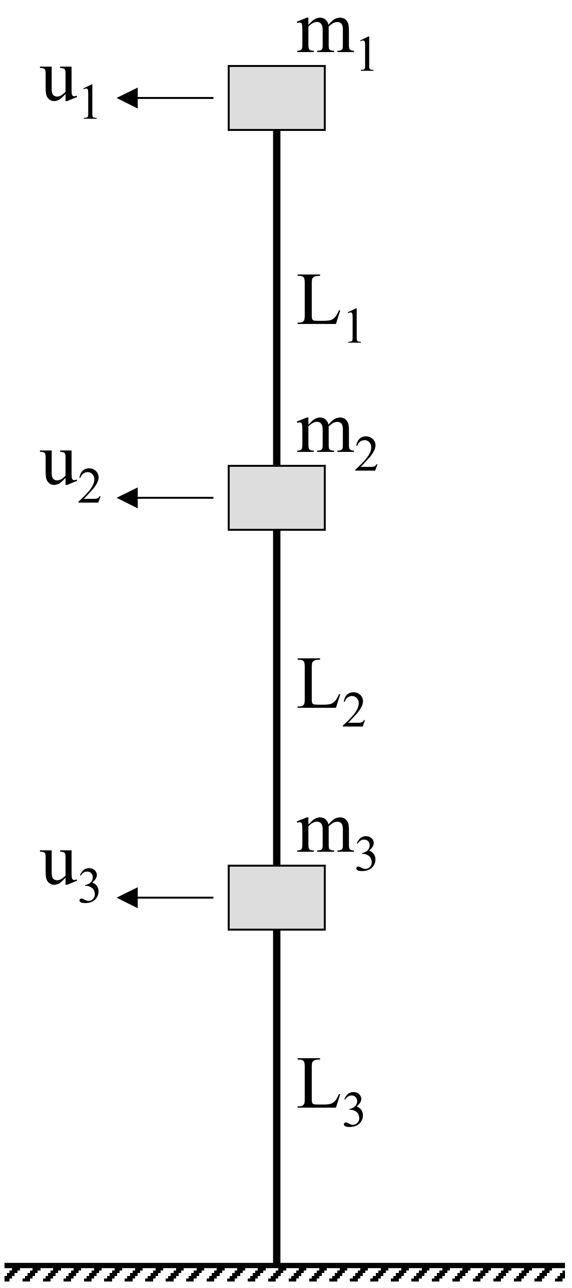
Given:

m_1
m_2
L_1
L_2
L_3
EI
ω_1
ω_2
ω_3

Solve for m_3



3 Story Bending Beam



Given:

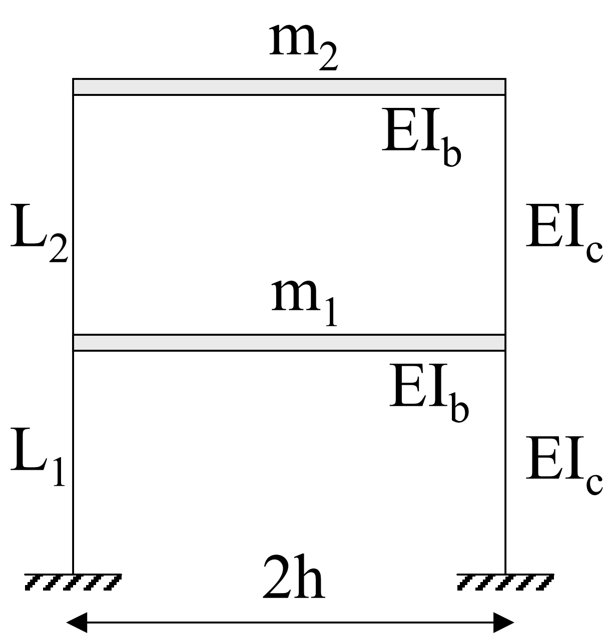
m_1
m_2
$L_1=L_2=L_3=L$
EI
ω_1
ω_2
ω_3

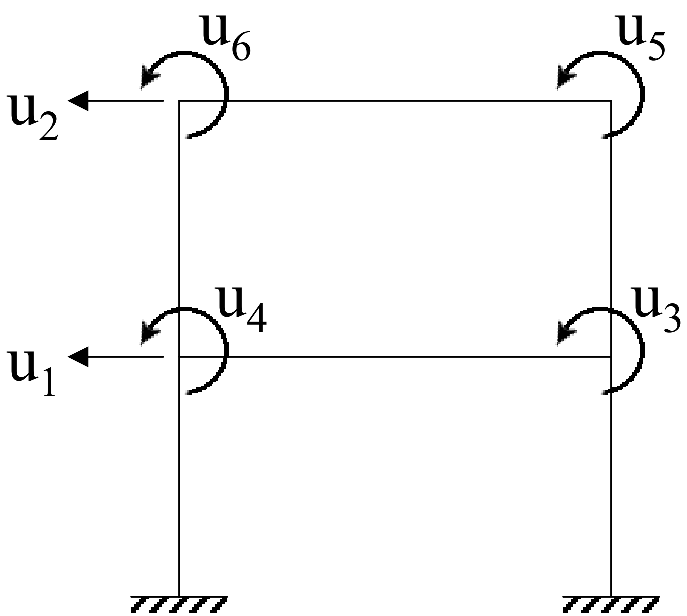
Solve for m_3

Part 1: Determining the unknown mass of your structure

Step 1: Assemble the mass and stiffness matrices for your structure

For Example: Find m_2, given m_1, $L_1 = L_2 = h$, EI_c, & EI_b





Step 1: Mass and Stiffness Matrices (continued)

Example (continued):

$$m = \begin{bmatrix} m_1 & 0 & 0 & 0 & 0 & 0 \\ 0 & m_2 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 \end{bmatrix}$$

$$k = \begin{bmatrix} \frac{48 \cdot E \cdot I_c}{h^3} & \frac{-24 \cdot E \cdot I_c}{h^3} & 0 & 0 & \frac{6 \cdot E \cdot I_c}{h^2} & \frac{6 \cdot E \cdot I_c}{h^2} \\ \frac{-24 \cdot E \cdot I_c}{h^3} & \frac{24 \cdot E \cdot I_c}{h^3} & \frac{-6 \cdot E \cdot I_c}{h^2} & \frac{-6 \cdot E \cdot I_c}{h^2} & \frac{-6 \cdot E \cdot I_c}{h^2} & \frac{-6 \cdot E \cdot I_c}{h^2} \\ 0 & \frac{-6 \cdot E \cdot I_c}{h^2} & \frac{8 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} & \frac{2 \cdot E \cdot I_c}{h} & 0 \\ 0 & \frac{-6 \cdot E \cdot I_c}{h^2} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} & \frac{8 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} & 0 & \frac{2 \cdot E \cdot I_c}{h} \\ \frac{6 \cdot E \cdot I_c}{h^2} & \frac{-6 \cdot E \cdot I_c}{h^2} & \frac{2 \cdot E \cdot I_c}{h} & 0 & \frac{4 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} \\ \frac{6 \cdot E \cdot I_c}{h^2} & \frac{-6 \cdot E \cdot I_c}{h^2} & 0 & \frac{2 \cdot E \cdot I_c}{h} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} & \frac{4 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} \end{bmatrix}$$

Determining the unknown mass of your structure

Step 2: Perform Static Condensation (if necessary)

$$[k] = \begin{bmatrix} k_{tt} & k_{t0} \\ k_{0t} & k_{00} \end{bmatrix} \qquad \hat{k}_{tt} = k_{tt} - k_{0t}^T k_{00}^{-1} k_{0t}$$

Example (continued):

$$k_{tt} = \begin{bmatrix} \frac{48 \cdot E \cdot I_c}{h^3} & \frac{-24 \cdot E \cdot I_c}{h^3} \\ \frac{-24 \cdot E \cdot I_c}{h^3} & \frac{24 \cdot E \cdot I_c}{h^3} \end{bmatrix} \qquad k_{0t} = \begin{bmatrix} 0 & -\frac{6 \cdot E \cdot I_c}{h^2} \\ 0 & -\frac{6 \cdot E \cdot I_c}{h^2} \\ \frac{6 \cdot E \cdot I_c}{h^2} & -\frac{6 \cdot E \cdot I_c}{h^2} \\ \frac{6 \cdot E \cdot I_c}{h^2} & -\frac{6 \cdot E \cdot I_c}{h^2} \end{bmatrix}$$

$$k_{00} = \begin{bmatrix} \frac{8 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} & \frac{2 \cdot E \cdot I_c}{h} & 0 \\ \frac{2 \cdot E \cdot I_b}{2 \cdot h} & \frac{8 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} & 0 & \frac{2 \cdot E \cdot I_c}{h} \\ \frac{2 \cdot E \cdot I_c}{h} & 0 & \frac{4 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} \\ 0 & \frac{2 \cdot E \cdot I_c}{h} & \frac{2 \cdot E \cdot I_b}{2 \cdot h} & \frac{4 \cdot E \cdot I_c}{h} + \frac{4 \cdot E \cdot I_b}{2 \cdot h} \end{bmatrix}$$

Step 2: Static Condensation (continued)

Example (continued):

$$\hat{k}_{tt} = k_{tt} - k_{0t}^{T} k_{00}^{-1} k_{0t} = \begin{bmatrix} 24 \cdot E \cdot Ic \cdot \dfrac{\left(32 \cdot Ic^2 + 63 \cdot Ic \cdot Ib + 18 \cdot Ib^2\right)}{\left[\left(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right) \cdot h^3\right]} & -24 \cdot E \cdot Ic \cdot \dfrac{\left(10 \cdot Ic^2 + 27 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right)}{\left[\left(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right) \cdot h^3\right]} \\ -24 \cdot E \cdot Ic \cdot \dfrac{\left(10 \cdot Ic^2 + 27 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right)}{\left[\left(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right) \cdot h^3\right]} & 24 \cdot E \cdot Ic \cdot \dfrac{\left(4 \cdot Ic^2 + 18 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right)}{\left[\left(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right) \cdot h^3\right]} \end{bmatrix}$$

Where $\hat{k}_{tt}$ is the condensed stiffness matrix

Determining the unknown mass of your structure

Step 3: Solve the Eigen-value problem to determine the determinant of

$$\left[[k]-\omega^2[m]\right]$$

Example (continued):

$$[k]-\omega^2[m] = \begin{bmatrix} 24\cdot E\cdot Ic\cdot\dfrac{\left(32\cdot Ic^2+63\cdot Ic\cdot Ib+18\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]}-\omega^2\cdot m1 & -24\cdot E\cdot Ic\cdot\dfrac{\left(10\cdot Ic^2+27\cdot Ic\cdot Ib+9\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]} \\ -24\cdot E\cdot Ic\cdot\dfrac{\left(10\cdot Ic^2+27\cdot Ic\cdot Ib+9\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]} & 24\cdot E\cdot Ic\cdot\dfrac{\left(4\cdot Ic^2+18\cdot Ic\cdot Ib+9\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]}-\omega^2\cdot m2 \end{bmatrix}$$

Step 3: Eigen-value Problem (continued)

Example (continued):

$$\left|\left[[k]-\omega^2[m]\right]\right| =$$

$$= \frac{\left(576 \cdot E^2 \cdot Ic^4 + 5184 \cdot Ic^3 \cdot E^2 \cdot Ib - 96 \cdot Ic^3 \cdot \omega^2 \cdot m1 \cdot h^3 \cdot E - 768 \cdot Ic^3 \cdot E \cdot \omega^2 \cdot m2 \cdot h^3 + 28 \cdot Ic^2 \cdot \omega^4 \cdot m1 \cdot h^6 \cdot m2 - 432 \cdot Ic^2 \cdot \omega^2 \cdot m1 \cdot h^3 \cdot E \cdot Ib + 5184 \cdot Ic^2 \cdot E^2 \cdot Ib^2 - 1512 \cdot Ic^2 \cdot E \cdot \omega^2 \cdot m2 \cdot h^3 \cdot Ib - 432 \cdot Ic \cdot E \cdot \omega^2 \cdot m2 \cdot h^3 \cdot Ib^2 - 216 \cdot Ic \cdot \omega^2 \cdot m1 \cdot h^3 \cdot E \cdot Ib^2 + 36 \cdot Ic \cdot \omega^4 \cdot m1 \cdot h^6 \cdot m2 \cdot Ib + 9 \cdot \omega^4 \cdot m1 \cdot h^6 \cdot m2 \cdot Ib^2\right)}{\left[h^6 \cdot \left(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2\right)\right]}$$

Determining the unknown mass of your structure

Step 4: Plug the given values for m_1, L_1, L_2, EI, & ω_1 into

$$\left| \left[[k] - \omega^2 [m] \right] \right| = 0,$$ and solve for the unknown mass.

Example (continued):

Given:

$m_1 = 0.2$ kg
$L_1 = 0.3048$ m
$L_2 = 0.3048$ m
$EI = 1.0889242$ n-m^2
$\omega_1 = 12.566$ rad/s
$\omega_2 = 86.013$rad/s

Step 4: Solve for unknown m (continued)

Example (continued):

Plug m_1, L_1, L_2, EI, & ω_1 into $\left|\left[k\right]-\omega^2\left[m\right]\right| = 0$

Solve for m_2

$$m_2 = 24\cdot E\cdot Ic\cdot\frac{\left(24\cdot E\cdot Ic^3 + 216\cdot Ic^2\cdot E\cdot Ib - 4\cdot Ic^2\cdot\omega_1^{\;2}\cdot m1\cdot h^3 - 18\cdot Ic\cdot\omega_1^{\;2}\cdot m1\cdot h^3\cdot Ib + 216\cdot Ic\cdot E\cdot Ib^2 - 9\cdot\omega_1^{\;2}\cdot m1\cdot h^3\cdot Ib^2\right)}{\left[h^3\cdot\omega_1^{\;2}\cdot\left(768\cdot E\cdot Ic^3 - 28\cdot Ic^2\cdot\omega_1^{\;2}\cdot m1\cdot h^3 + 1512\cdot Ic^2\cdot E\cdot Ib + 432\cdot Ic\cdot E\cdot Ib^2 - 36\cdot Ic\cdot\omega_1^{\;2}\cdot m1\cdot h^3\cdot Ib - 9\cdot\omega_1^{\;2}\cdot m1\cdot h^3\cdot Ib^2\right)\right]}$$

$$m_2 = 0.949 \text{ kg}$$

Determining the unknown mass of your structure

Step 5: Perform a self-check on the value of the mass you just found

Plug the newly determined mass along with the remaining ω_i into $\left|\left[[k]-\omega^2[m]\right]\right|$ and verify that the determinant is still equal to zero

Example (continued):

$$m_2 = 24 \cdot E \cdot Ic \cdot \frac{\left(24 \cdot E \cdot Ic^3 + 216 \cdot Ic^2 \cdot E \cdot Ib - 4 \cdot Ic^2 \cdot \omega_2^{\,2} \cdot m1 \cdot h^3 - 18 \cdot Ic \cdot \omega_2^{\,2} \cdot m1 \cdot h^3 \cdot Ib + 216 \cdot Ic \cdot E \cdot Ib^2 - 9 \cdot \omega_2^{\,2} \cdot m1 \cdot h^3 \cdot Ib^2\right)}{\left[h^3 \cdot \omega_2^{\,2} \cdot \left(768 \cdot E \cdot Ic^3 - 28 \cdot Ic^2 \cdot \omega_2^{\,2} \cdot m1 \cdot h^3 + 1512 \cdot Ic^2 \cdot E \cdot Ib + 432 \cdot Ic \cdot E \cdot Ib^2 - 36 \cdot Ic \cdot \omega_2^{\,2} \cdot m1 \cdot h^3 \cdot Ib - 9 \cdot \omega_2^{\,2} \cdot m1 \cdot h^3 \cdot Ib^2\right)\right]}$$

$m_2 = 0.949$ kg (same as with ω_1 – OK!)

Part 2: Modal Analysis

Step 1: Determine the Mode Shapes ϕ_n

$$\left(\mathbf{k} - \omega^2 \mathbf{m}\right)\mathbf{u} = \mathbf{0} \qquad \text{Equation 1}$$

Continuing with the Eigen-value problem solution (again, Matlab does this, or by hand for a 2-dof system), for each ω_n we get an associated ϕ_n ←mode shape. To do this (for each identified ω_n), go ahead and substitute this ω_n for ω in Eq. 1 above. Upon this substitution, you can solve for the corresponding vector **u**, the components of which defines the mode shape ϕ_n .

Step 1: Mode Shapes (continued)

For the Previous example:

$$\omega_1 = 12.566 \text{ rad/s}$$

$$\begin{bmatrix} 24\cdot E\cdot Ic\cdot\frac{\left(32\cdot Ic^2+63\cdot Ic\cdot Ib+18\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]}-\omega 1^2\cdot m1 & -24\cdot E\cdot Ic\cdot\frac{\left(10\cdot Ic^2+27\cdot Ic\cdot Ib+9\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]} \\ -24\cdot E\cdot Ic\cdot\frac{\left(10\cdot Ic^2+27\cdot Ic\cdot Ib+9\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]} & 24\cdot E\cdot Ic\cdot\frac{\left(4\cdot Ic^2+18\cdot Ic\cdot Ib+9\cdot Ib^2\right)}{\left[\left(28\cdot Ic^2+36\cdot Ic\cdot Ib+9\cdot Ib^2\right)\cdot h^3\right]}-\omega 1^2\cdot m2 \end{bmatrix} = \begin{bmatrix} 1.397\cdot 10^3 & -581.566 \\ -581.566 & 242.065 \end{bmatrix}$$

$$\begin{bmatrix} 1.397\cdot 10^3 & -581.566 \\ -581.566 & 242.065 \end{bmatrix}\cdot\begin{bmatrix} \phi\ 11 \\ \phi\ 21 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Let $\phi_{11} = 1.0$; therefore, $\phi_{21} = 2.402$

Step 1: Mode Shapes (continued)

Example (continued):

$$\omega_2 = 86.011 \text{ rad/s}$$

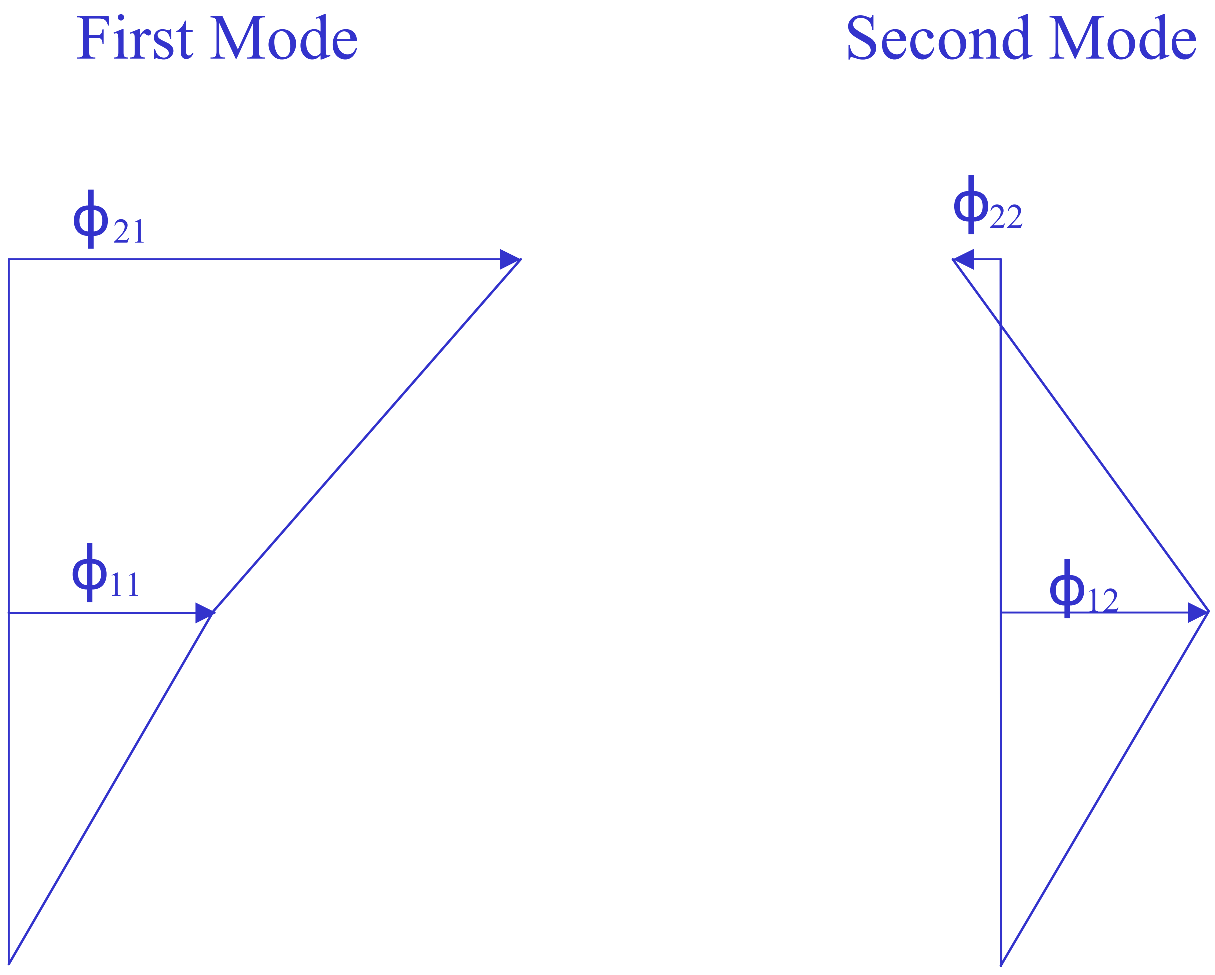
$$\begin{bmatrix} 24 \cdot E \cdot Ic \cdot \frac{(32 \cdot Ic^2 + 63 \cdot Ic \cdot Ib + 18 \cdot Ib^2)}{[(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2) \cdot h^3]} - \omega 2^2 \cdot m1 & -24 \cdot E \cdot Ic \cdot \frac{(10 \cdot Ic^2 + 27 \cdot Ic \cdot Ib + 9 \cdot Ib^2)}{[(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2) \cdot h^3]} \\ -24 \cdot E \cdot Ic \cdot \frac{(10 \cdot Ic^2 + 27 \cdot Ic \cdot Ib + 9 \cdot Ib^2)}{[(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2) \cdot h^3]} & 24 \cdot E \cdot Ic \cdot \frac{(4 \cdot Ic^2 + 18 \cdot Ic \cdot Ib + 9 \cdot Ib^2)}{[(28 \cdot Ic^2 + 36 \cdot Ic \cdot Ib + 9 \cdot Ib^2) \cdot h^3]} - \omega 2^2 \cdot m2 \end{bmatrix} = \begin{bmatrix} -50.933 & -581.566 \\ -581.566 & -6.629 \cdot 10^3 \end{bmatrix}$$

$$\begin{bmatrix} -50.933 & -581.566 \\ -581.566 & -6.629 \cdot 10^3 \end{bmatrix} \cdot \begin{bmatrix} \phi_{12} \\ \phi_{22} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

Let $\phi_{12} = 1.0$; therefore, $\phi_{22} = -0.0876$

Step 1: Mode Shapes (continued)

Example (continued):



Part 2: Modal Analysis

Step 2: Determine the Modal Participation Factors $\frac{L_i}{M_i}$

$$M_i = \sum_{j=1}^{NDOF} m_j \phi_{ji}^2$$

$$L_i = \sum_{j=1}^{NDOF} m_j \phi_{ji}$$

$$\phi_i = \begin{bmatrix} \phi_{1i} \\ \phi_{2i} \\ . \\ . \\ . \\ \phi_{NDOF\ i} \end{bmatrix}$$

Step 2: Modal Participation Factors (continued)

Example (continued):

$$M_i = \sum_{j=1}^{NDOF} m_j \phi_{ji}^2$$

$$M_1 = m_1\phi_{11}^2 + m_2\phi_{21}^2 = (0.2)(1)^2 + (0.949)(2.402)^2 = 5.67535 \text{ kg}$$

$$M_2 = m_1\phi_{12}^2 + m_2\phi_{22}^2 = (0.2)(1)^2 + (0.949)(-0.0876)^2 = 0.20728 \text{ kg}$$

$$L_i = \sum_{j=1}^{NDOF} m_j \phi_{ji}$$

$$L_1 = m_1\phi_{11} + m_2\phi_{21} = (0.2)(1) + (0.949)(2.402) = 2.4795 \text{ kg}$$

$$L_2 = m_1\phi_{12} + m_2\phi_{22} = (0.2)(1) + (0.949)(-0.0876) = 0.11687 \text{ kg}$$

Step 2: Modal Participation Factors (continued)

Example (continued):

$$\frac{L_1}{M_1} = \frac{2.4795 \text{ kg}}{5.67535 \text{ kg}} = 0.437$$

$$\frac{L_2}{M_2} = \frac{0.11687 \text{ kg}}{0.20728 \text{ kg}} = 0.563$$

Part 2: Modal Analysis

Step 3: Determine K_i

$$K_i = \omega_i^2 M_i$$

$$K_1 = \omega_1^2 M_1 = (12.566)^2 (5.67535) = 896.1625$$

$$K_2 = \omega_2^2 M_2 = (86.011)^2 (0.20728) = 1533.435$$

Part 2: Modal Analysis

Step 4: Add Damping

Now, you can add any modal damping you wish (which is another big plus, since you control the damping in each mode individually). If you choose ζ_i = 0.02 or 0.05, the equations become:

$$\ddot{q}_i + 2\xi_i \omega_i \dot{q}_i + \omega_i^2 q_i = -\frac{L_i}{M_i}\ddot{u}_g \, , \quad i = 1, 2, \ldots \text{ NDOF}$$

Part 2: Modal Analysis

Step 5: Solve for $q_i(t)$

Solve for $q_i(t)$ in the above uncoupled equations (using a SDOF-type program), and the final solution is obtained from:

$$\mathbf{u} = \mathbf{\Phi q}$$
$$\dot{\mathbf{u}} = \mathbf{\Phi}\dot{\mathbf{q}}$$
$$\ddot{\mathbf{u}} = \mathbf{\Phi}\ddot{\mathbf{q}}$$
$$\ddot{\mathbf{u}}^t = \ddot{\mathbf{u}} + \mathbf{1}\ddot{u}_g$$

Step 5: Solve for $q_i(t)$ (continued)

We will solve for $q_i(t)$ using a modified version of the spreadsheet for solving for the response of a SDOF system using Newmark's Method

Part 3: Spreadsheet for Modal Analysis

Step-By-Step Procedure For Setting Up A Spreadsheet For Using Newmark's Method and Modal Analysis To Solve For The Response Of A Multi-Degree Of Freedom (MDOF) System

Start with the equation of motion for a linear multi-degree of freedom system with base ground excitation:

$$\mathbf{m}\ddot{\mathbf{u}} + \mathbf{c}\dot{\mathbf{u}} + \mathbf{k}\mathbf{u} = -\mathbf{m}\mathbf{1}\ddot{u}_g$$

Using Modal Analysis, we can rewrite the original coupled matrix equation of motion as a set of un-coupled equations.

$$\ddot{q}_i + 2\zeta\omega\dot{q}_i + \omega_i^2 q_i = -\frac{L_i}{M_i}\ddot{u}_g \quad , \; i = 1, 2, \ldots, NDOF$$

with initial conditions of $d_i(t=0) = d_{io}$ and $v_i(t=0) = v_{io}$

Note that total acceleration or absolute acceleration will be

$$\ddot{q}_{i\,abs} = \ddot{q}_i + \ddot{u}_g$$

We can solve each one separately (as a SDOF system), and compute histories of q_i and their time derivatives. To compute the system response, plug the q vector back into $\mathbf{u} = \mathbf{\Phi q}$ and get the u vector (and the same for the time derivatives to get velocity and acceleration).

The beauty here is that there is no matrix operations involved, since the matrix equation of motion has become a set of un-coupled equation, each including only one generalized coordinate q_n .

In the spreadsheet, we will solve each mode in a separate worksheet.

Step 1 - Define System Properties and Initial Conditions for First Mode

(A) Begin by setting up the cells for the Mass, Stiffness, and Damping of the SDOF System (Fig. 1). These values are known.

(B) Set up the cells for the modal participation factor $\frac{L_i}{M_i}$ and mode shape ϕ_i (Fig. 1). These values must be determined in advance using Modal Analysis.

(C) Calculate the Natural Frequency of the SDOF system using the equation

$$\omega_i = \sqrt{K_i / M_i} \qquad \text{(Equation 1)}$$

Step 1 (continued)

Note: If the system damping is given in terms of the Modal Damping Ratio (ζ_i) then the Damping (C_i) can be calculated using the equation:

$$C_i = 2\,\zeta_i\,\omega_i\,M_i \qquad \text{(Equation 2)}$$

(D) Set up the cells for the 2 Newmark Coefficients α & β (Fig. 1), which will allow for performing

a) the Average Acceleration Method, use $\alpha = \frac{1}{2}$ and $\beta = \frac{1}{6}$.

b) the Linear Acceleration Method, use $\alpha = \frac{1}{2}$ and $\beta = \frac{1}{4}$.

(E) Set up cells (Fig. 1) for the initial displacement and velocity (d_o and v_o respectively)

Step 1 (continued)

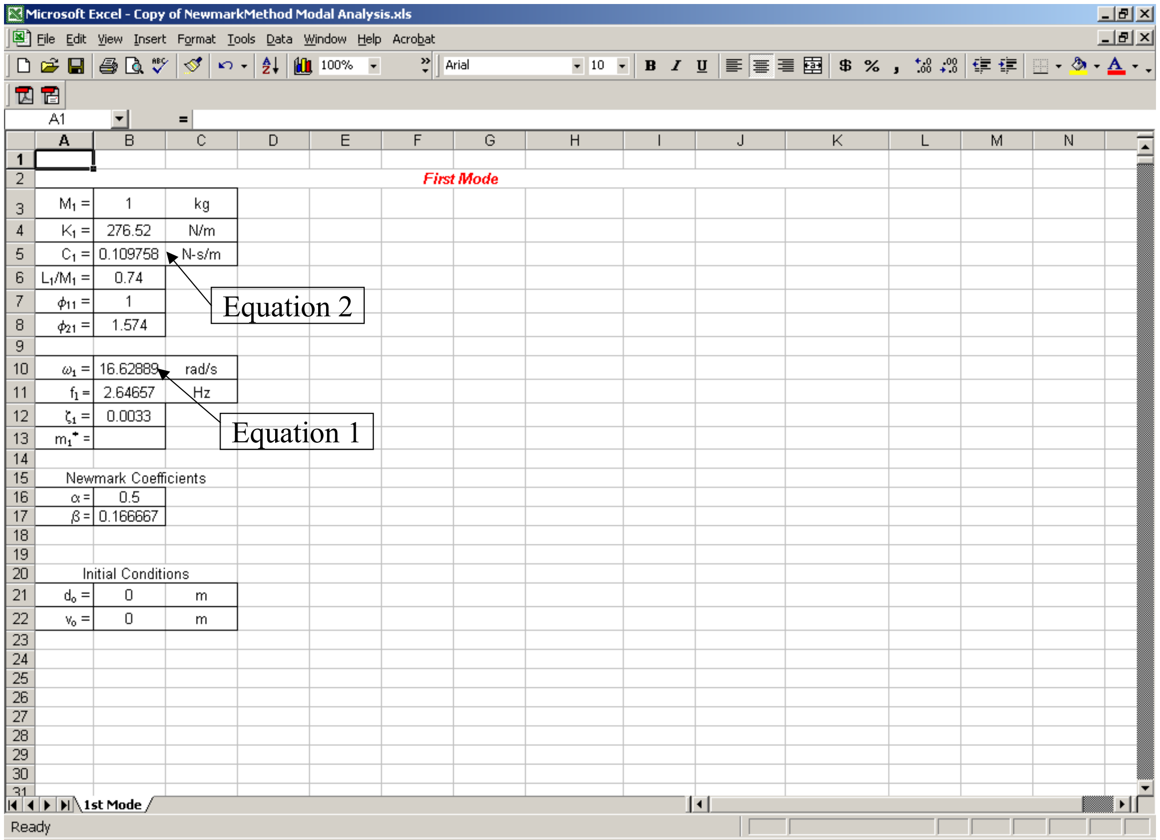


Figure 1: Spreadsheet After Completing Step 1

Step 2 – Set Up Columns for Solving The Equation of Motion Using Newmark's Method

Place a cell (Fig. 2) for the time increment (Δt).

Place columns (Fig. 2) for the time, base excitation, applied force divided by mass, relative acceleration, relative velocity, and relative displacement.

Step 2 (continued)

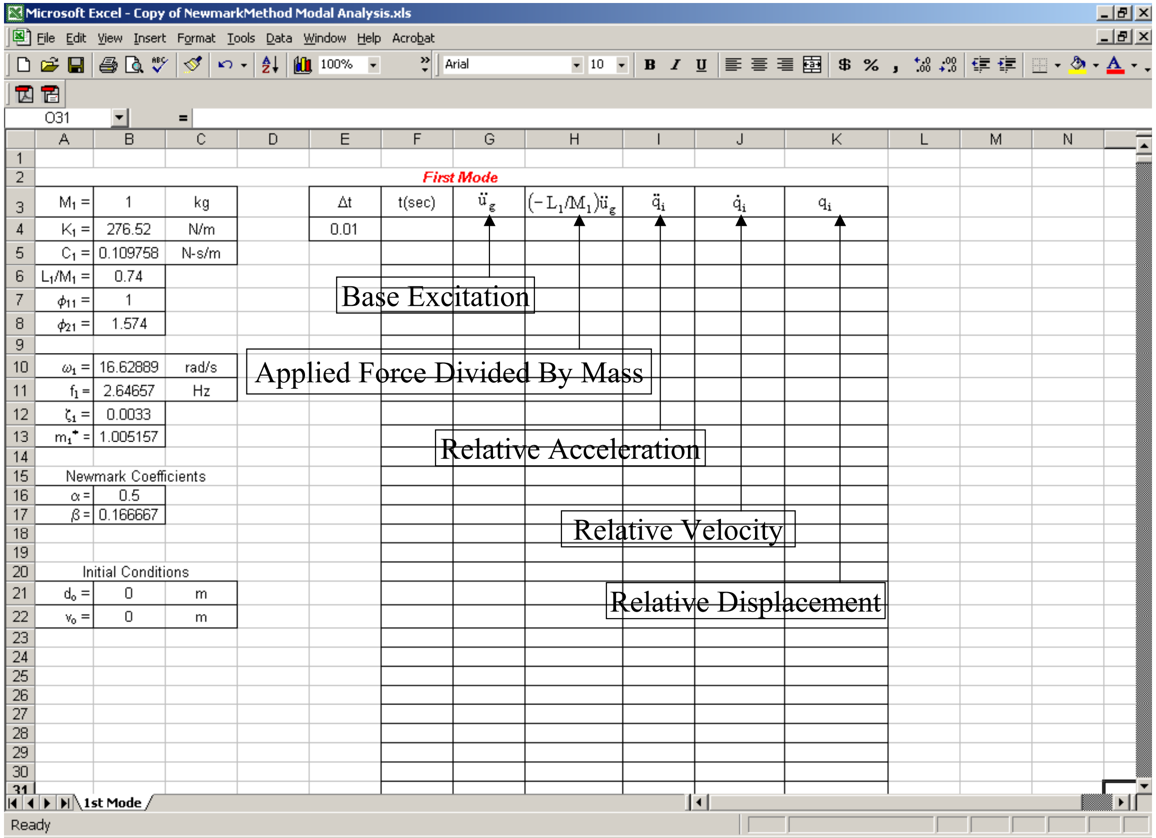


Figure 2: Spreadsheet After Completing Step 2

Step 3 – Enter the Time t & Applied Force f(t) into the Spreadsheet

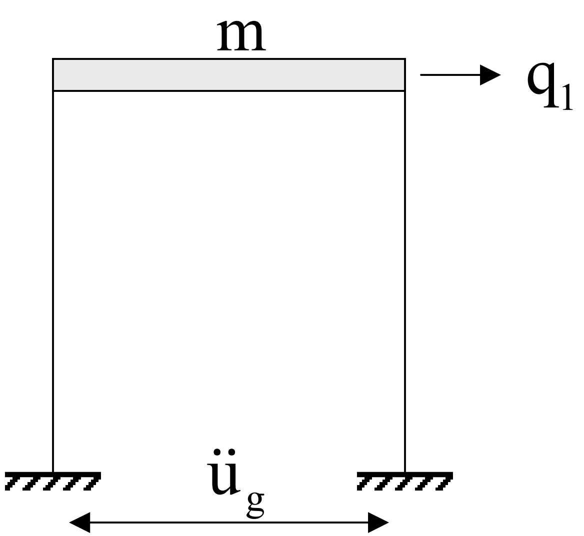
$$t_{i+1} = t_i + \Delta t$$ (Equation 3) (Fig. 3)

For the earthquake problem (acceleration applied to base of the structure), the applied force divided by the mass is calculated using:

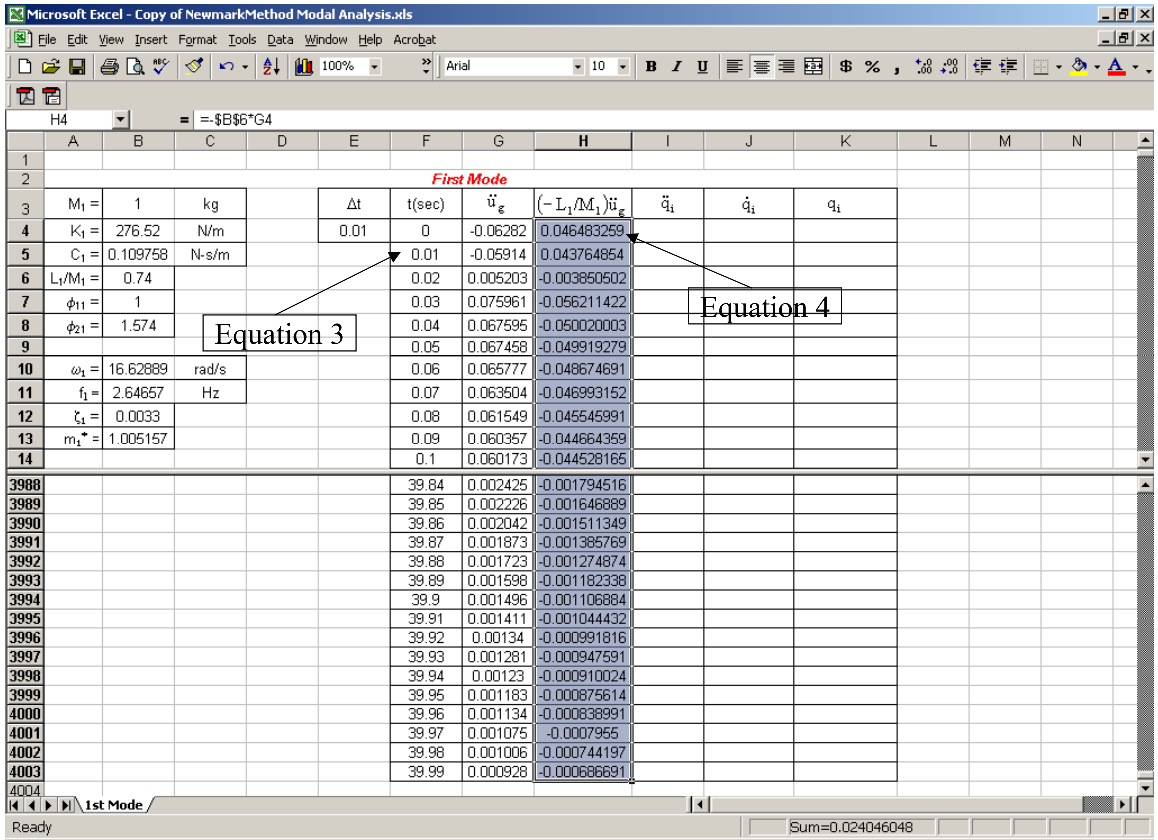
$$\frac{f_i(t)}{M_1} = -\frac{L_1}{M_1}\ddot{u}_{g_i}$$ (Equation 4) (Fig. 3)

where, $\ddot{u}_{g_i}$ is the applied base acceleration at step i. (Typically this is the base excitation time history)

Check the units of the input motion file. They must be compatible with the units of the mass, stiffness, and damping!



Step 3 (continued)



	A	B	C	D	E	F	G	H	I	J	K
1											
2						*First Mode*					
3	M_1 =	1	kg		Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
4	K_1 =	276.52	N/m		0.01	0	-0.06282	0.046483259			
5	C_1 =	0.109758	N-s/m			0.01	-0.05914	0.043764854			
6	L_1/M_1 =	0.74				0.02	0.005203	-0.003850502			
7	ϕ_{11} =	1				0.03	0.075961	-0.056211422			
8	ϕ_{21} =	1.574				0.04	0.067595	-0.050020003			
9						0.05	0.067458	-0.049919279			
10	ω_1 =	16.62889	rad/s			0.06	0.065777	-0.048674691			
11	f_1 =	2.64657	Hz			0.07	0.063504	-0.046993152			
12	ζ_1 =	0.0033				0.08	0.061549	-0.045545991			
13	m_1^* =	1.005157				0.09	0.060357	-0.044664359			
14						0.1	0.060173	-0.044528165			
3988						39.84	0.002425	-0.001794516			
3989						39.85	0.002226	-0.001646889			
3990						39.86	0.002042	-0.001511349			
3991						39.87	0.001873	-0.001385769			
3992						39.88	0.001723	-0.001274874			
3993						39.89	0.001598	-0.001182338			
3994						39.9	0.001496	-0.001106884			
3995						39.91	0.001411	-0.001044432			
3996						39.92	0.00134	-0.000991816			
3997						39.93	0.001281	-0.000947591			
3998						39.94	0.00123	-0.000910024			
3999						39.95	0.001183	-0.000875614			
4000						39.96	0.001134	-0.000838991			
4001						39.97	0.001075	-0.0007955			
4002						39.98	0.001006	-0.000744197			
4003						39.99	0.000928	-0.000686691			
4004											

Figure 3: Spreadsheet After Completing Step 3

Step 4 – Compute Initial Values of the Relative Acceleration, Relative Velocity, Relative Displacement, and Absolute Acceleration

(A) The Initial Relative Displacement and Relative Velocity are known from the initial conditions (Fig. 4).

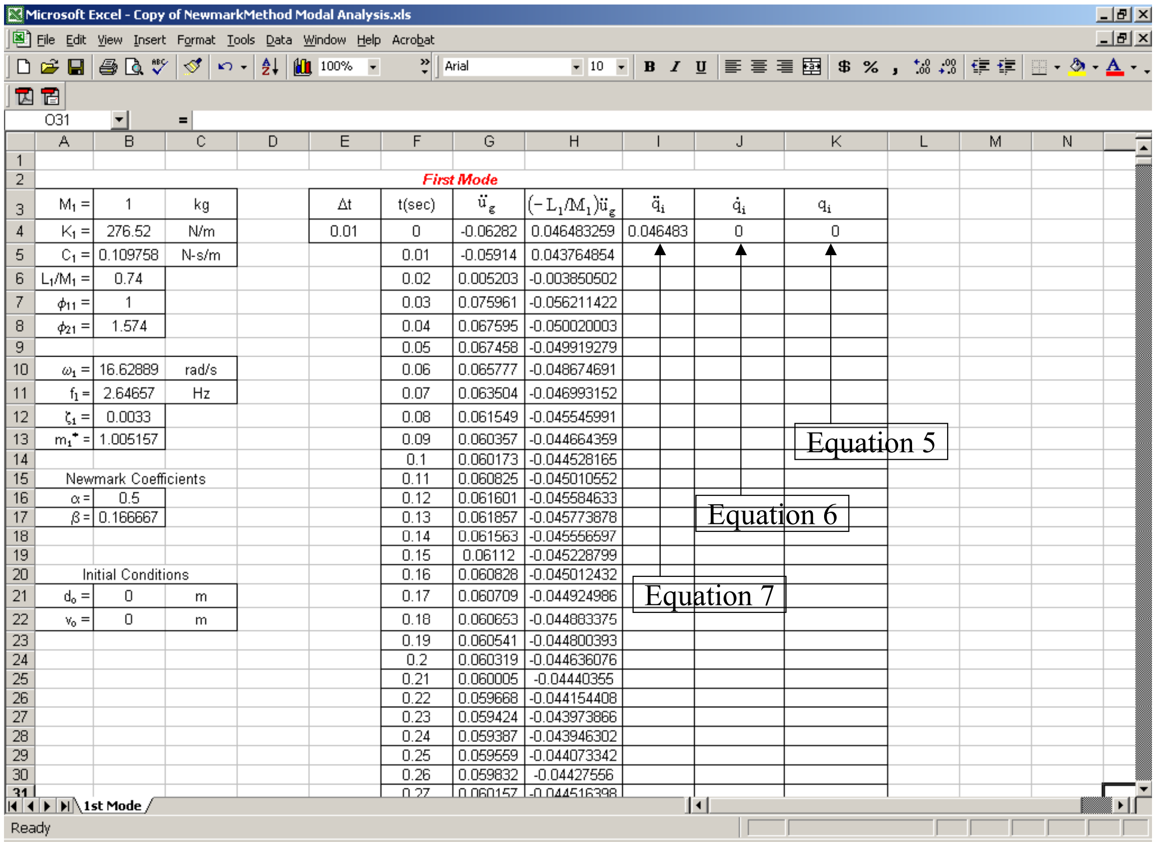
$$q(t=0) = d_o \qquad \text{(Equation 5)}$$

$$\dot{q}(t=0) = v_o \qquad \text{(Equation 6)}$$

(B) The Initial Relative Acceleration (Fig. 4) is calculated using

$$\ddot{q}(t=0) = -\frac{Li}{Mi}\ddot{u}_g - 2\zeta\omega v_o - \omega^2 d_o \qquad \text{(Equation 7)}$$

Step 4 (continued)



Microsoft Excel - Copy of NewmarkMethod Modal Analysis.xls

First Mode

M_1 =	1	kg
K_1 =	276.52	N/m
C_1 =	0.109758	N-s/m
L_1/M_1 =	0.74	
ϕ_{11} =	1	
ϕ_{21} =	1.574	
ω_1 =	16.62889	rad/s
f_1 =	2.64657	Hz
ζ_1 =	0.0033	
m_1^* =	1.005157	
Newmark Coefficients		
α =	0.5	
β =	0.166667	
Initial Conditions		
d_o =	0	m
v_o =	0	m

Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
0.01	0	-0.06282	0.046483259	0.046483	0	0
	0.01	-0.05914	0.043764854			
	0.02	0.005203	-0.003850502			
	0.03	0.075961	-0.056211422			
	0.04	0.067595	-0.050020003			
	0.05	0.067458	-0.049919279			
	0.06	0.065777	-0.048674691			
	0.07	0.063504	-0.046993152			
	0.08	0.061549	-0.045545991			
	0.09	0.060357	-0.044664359			
	0.1	0.060173	-0.044528165			
	0.11	0.060825	-0.045010552			
	0.12	0.061601	-0.045584633			
	0.13	0.061857	-0.045773878			
	0.14	0.061563	-0.045556597			
	0.15	0.06112	-0.045228799			
	0.16	0.060828	-0.045012432			
	0.17	0.060709	-0.044924986			
	0.18	0.060653	-0.044883375			
	0.19	0.060541	-0.044800393			
	0.2	0.060319	-0.044636076			
	0.21	0.060005	-0.04440355			
	0.22	0.059668	-0.044154408			
	0.23	0.059424	-0.043973866			
	0.24	0.059387	-0.043946302			
	0.25	0.059559	-0.044073342			
	0.26	0.059832	-0.04427556			
	0.27	0.060157	-0.044516398			

Figure 4: Spreadsheet After Completing Step 4

Step 5 – Compute Incremental Values of the Relative Acceleration, Relative Velocity, Relative Displacement, and Absolute Acceleration At Each Time Step (Fig. 5)

(A)

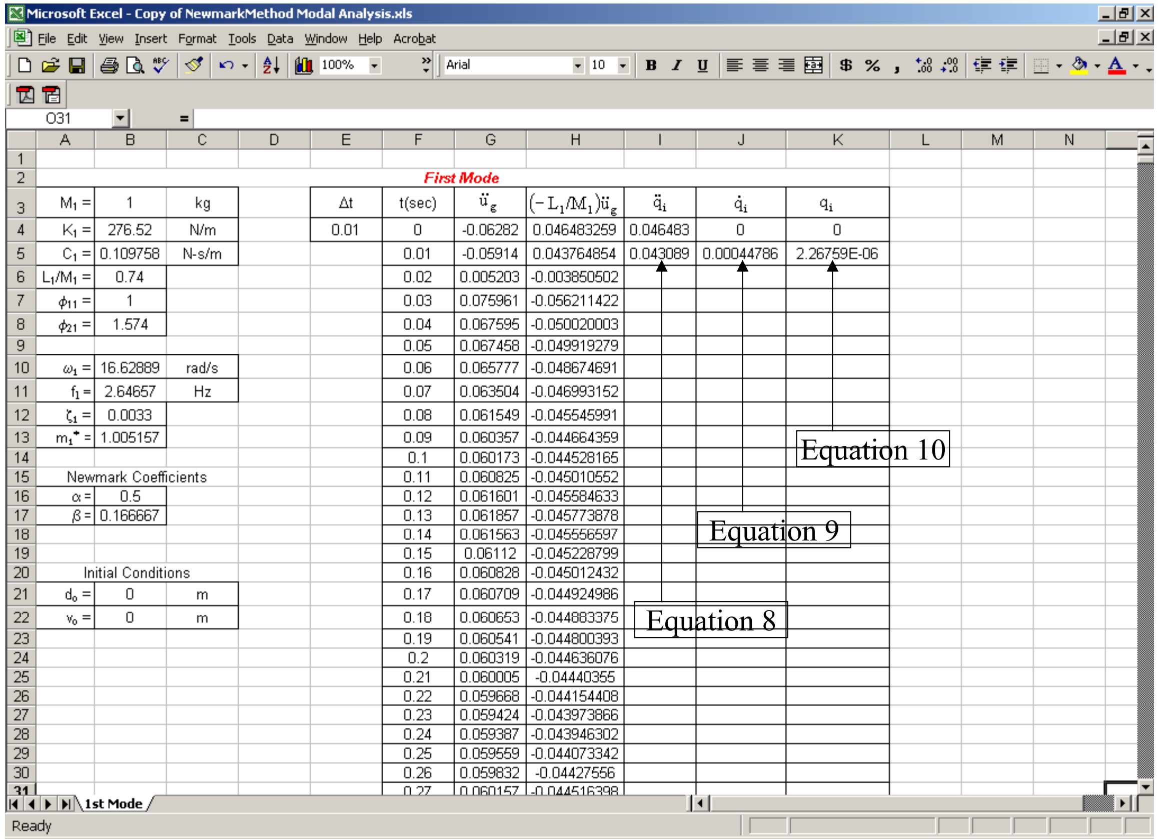
$$\ddot{q}_{i+1} = \frac{\left[-\frac{L_1}{M_1}\ddot{u}_{g_{i+1}} - C_1\left(\frac{\Delta t}{2}\ddot{q}_i + \dot{q}_i\right) - K_1\left(\frac{1}{2}\Delta t^2(1-2\beta)\ddot{q}_i + \Delta t\dot{q}_i + q_i\right)\right]}{m_1{*}} \quad \text{(Equation 8)}$$

$$\dot{q}_{i+1} = \ddot{q}_i\Delta t(1-\alpha) + \ddot{q}_{i+1}\Delta t\alpha + \dot{q}_i \quad \text{(Equation 9)}$$

$$q_{i+1} = \ddot{q}_i\frac{\Delta t^2}{2}(1-2\beta) + \ddot{q}_{i+1}\Delta t^2\beta + \dot{q}_i\Delta t + q_i \quad \text{(Equation 10)}$$

Where, the effective mass, $m_1{*} = M_1 + C_1\Delta t\alpha + K_1\Delta t^2\beta$

Step 5 (continued)



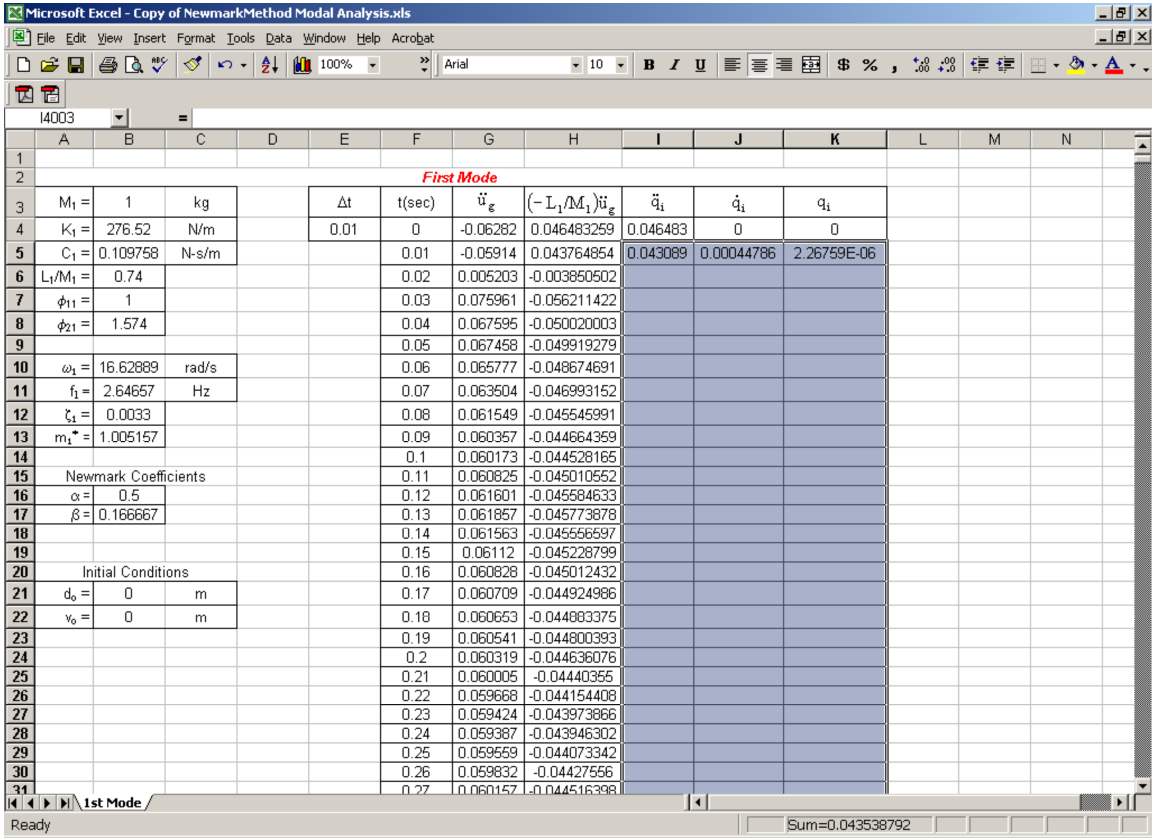
	A	B	C	E	F	G	H	I	J	K
2					First Mode					
3	M_1 =	1	kg	Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
4	K_1 =	276.52	N/m	0.01	0	-0.06282	0.046483259	0.046483	0	0
5	C_1 =	0.109758	N-s/m		0.01	-0.05914	0.043764854	0.043089	0.00044786	2.26759E-06
6	L_1/M_1 =	0.74			0.02	0.005203	-0.003850502			
7	ϕ_{11} =	1			0.03	0.075961	-0.056211422			
8	ϕ_{21} =	1.574			0.04	0.067595	-0.050020003			
9					0.05	0.067458	-0.049919279			
10	ω_1 =	16.62889	rad/s		0.06	0.065777	-0.048674691			
11	f_1 =	2.64657	Hz		0.07	0.063504	-0.046993152			
12	ζ_1 =	0.0033			0.08	0.061549	-0.045545991			
13	m_1^* =	1.005157			0.09	0.060357	-0.044664359			
14					0.1	0.060173	-0.044528165			
15	Newmark Coefficients				0.11	0.060825	-0.045010552			
16	α =	0.5			0.12	0.061601	-0.045584633			
17	β =	0.166667			0.13	0.061857	-0.045773878			
18					0.14	0.061563	-0.045556597			
19					0.15	0.06112	-0.045228799			
20	Initial Conditions				0.16	0.060828	-0.045012432			
21	d_o =	0	m		0.17	0.060709	-0.044924986			
22	v_o =	0	m		0.18	0.060653	-0.044883375			
23					0.19	0.060541	-0.044800393			
24					0.2	0.060319	-0.044636076			
25					0.21	0.060005	-0.04440355			
26					0.22	0.059668	-0.044154408			
27					0.23	0.059424	-0.043973866			
28					0.24	0.059387	-0.043946302			
29					0.25	0.059559	-0.044073342			
30					0.26	0.059832	-0.04427556			
31					0.27	0.060157	-0.044516398			

Figure 5: Spreadsheet with values for the Relative Acceleration, Relative Velocity, and Relative Displacement at Time Step 1

Step 5 (continued)

(B) Then, highlight columns I, J, & K and rows 4 through to the last time step (in this example 4003) and "Fill Down" (Ctrl+D).

See Figures 6 and 7.



	A	B	C	D	E	F	G	H	I	J	K
1											
2						First Mode					
3	M_1 =	1	kg		Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
4	K_1 =	276.52	N/m		0.01	0	-0.06282	0.046483259	0.046483	0	0
5	C_1 =	0.109758	N-s/m			0.01	-0.05914	0.043764854	0.043089	0.00044786	2.26759E-06
6	L_1/M_1 =	0.74				0.02	0.005203	-0.003850502			
7	ϕ_{11} =	1				0.03	0.075961	-0.056211422			
8	ϕ_{21} =	1.574				0.04	0.067595	-0.050020003			
9						0.05	0.067458	-0.049919279			
10	ω_1 =	16.62889	rad/s			0.06	0.065777	-0.048674691			
11	f_1 =	2.64657	Hz			0.07	0.063504	-0.046993152			
12	ζ_1 =	0.0033				0.08	0.061549	-0.045545991			
13	m_1^* =	1.005157				0.09	0.060357	-0.044664359			
14						0.1	0.060173	-0.044528165			
15	Newmark Coefficients					0.11	0.060825	-0.045010552			
16	α =	0.5				0.12	0.061601	-0.045584633			
17	β =	0.166667				0.13	0.061857	-0.045773878			
18						0.14	0.061563	-0.045556597			
19						0.15	0.06112	-0.045228799			
20	Initial Conditions					0.16	0.060828	-0.045012432			
21	d_o =	0	m			0.17	0.060709	-0.044924986			
22	v_o =	0	m			0.18	0.060653	-0.044883375			
23						0.19	0.060541	-0.044800393			
24						0.2	0.060319	-0.044636076			
25						0.21	0.060005	-0.04440355			
26						0.22	0.059668	-0.044154408			
27						0.23	0.059424	-0.043973866			
28						0.24	0.059387	-0.043946302			
29						0.25	0.059559	-0.044073342			
30						0.26	0.059832	-0.04427556			
31						0.27	0.060157	-0.044516398			

Figure 6: Highlighted Cells

Step 5 (continued)

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I5 = =(H5-$B$5*($E$4/2*I4+J4)-$B$4*($E$4^2/2*(1-2*$B$17)*I4+$E$4*J4+K4))/$B$13

	A	B	C	D	E	F	G	H	I	J	K
1											
2						***First Mode***					
3	M_1 =	1	kg		Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
4	K_1 =	276.52	N/m		0.01	0	-0.06282	0.046483259	0.046483	0	0
5	C_1 =	0.109758	N-s/m			0.01	-0.05914	0.043764854	0.043089	0.00044786	2.26759E-06
6	L_1/M_1 =	0.74				0.02	0.005203	-0.003850502	-0.00615	0.00063253	8.0799E-06
7	ϕ_{11} =	1				0.03	0.075961	-0.056211422	-0.0599	0.00030229	1.32018E-05
8	ϕ_{21} =	1.574				0.04	0.067595	-0.050020003	-0.05368	-0.00026558	1.33335E-05
9						0.05	0.067458	-0.049919279	-0.05205	-0.00079422	8.02099E-06
10	ω_1 =	16.62889	rad/s			0.06	0.065777	-0.048674691	-0.04785	-0.00129374	-2.45378E-06
11	f_1 =	2.64657	Hz			0.07	0.063504	-0.046993152	-0.04191	-0.00174257	-1.76849E-05
12	ζ_1 =	0.0033				0.08	0.061549	-0.045545991	-0.03506	-0.00212741	-3.70919E-05
13	m_1^* =	1.005157				0.09	0.060357	-0.044664359	-0.02781	-0.00244172	-5.99979E-05
14						0.1	0.060173	-0.044528165	-0.02054	-0.00268345	-8.56843E-05
3989						39.85	0.002226	-0.001646889	-0.63618	0.10993343	0.002251087
3990						39.86	0.002042	-0.001511349	-0.92903	0.10210736	0.003313732
3991						39.87	0.001873	-0.001385769	-1.19601	0.09148214	0.004283904
3992						39.88	0.001723	-0.001274874	-1.42981	0.07835302	0.005135028
3993						39.89	0.001598	-0.001182338	-1.62404	0.06308374	0.00584383
3994						39.9	0.001496	-0.001106884	-1.7734	0.04609653	0.006390976
3995						39.91	0.001411	-0.001044432	-1.87382	0.02786044	0.006761598
3996						39.92	0.00134	-0.000991816	-1.92259	0.00887839	0.006945699
3997						39.93	0.001281	-0.000947591	-1.91843	-0.0103267	0.006938422
3998						39.94	0.00123	-0.000910024	-1.8615	-0.02922632	0.006740183
3999						39.95	0.001183	-0.000875614	-1.75342	-0.04730093	0.006356646
4000						39.96	0.001134	-0.000838991	-1.59723	-0.06405419	0.005798569
4001						39.97	0.001075	-0.0007955	-1.39726	-0.07902662	0.005081498
4002						39.98	0.001006	-0.000744197	-1.15906	-0.0918082	0.004225339
4003						39.99	0.000928	-0.000686691	-0.88923	-0.10204963	0.003253802
4004											
4005											

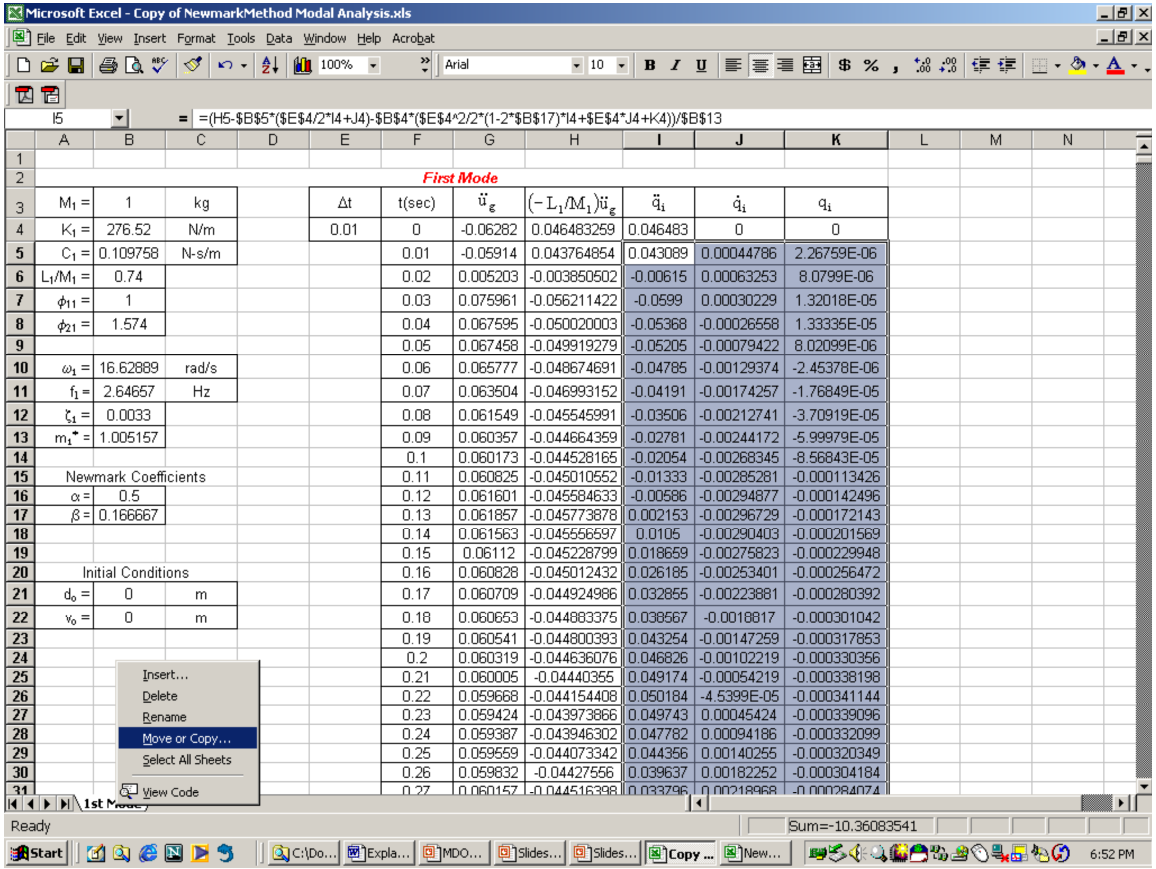
1st Mode

Ready Sum=-10.36083541

Figure 7: Spreadsheet After “Filling Down” Columns I through K

Step 6 – Create Additional Worksheet for Second Mode

Make a copy of the "1st Mode" worksheet by right clicking on the "1st Mode" tab and selecting "Move or Copy" (Fig. 8)



	A	B	C	E	F	G	H	I	J	K
1										
2						First Mode				
3	M_1 =	1	kg	Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
4	K_1 =	276.52	N/m	0.01	0	-0.06282	0.046483259	0.046483	0	0
5	C_1 =	0.109758	N-s/m		0.01	-0.05914	0.043764854	0.043089	0.00044786	2.26759E-06
6	L_1/M_1 =	0.74			0.02	0.005203	-0.003850502	-0.00615	0.00063253	8.0799E-06
7	ϕ_{11} =	1			0.03	0.075961	-0.056211422	-0.0599	0.00030229	1.32018E-05
8	ϕ_{21} =	1.574			0.04	0.067595	-0.050020003	-0.05368	-0.00026558	1.33335E-05
9					0.05	0.067458	-0.049919279	-0.05205	-0.00079422	8.02099E-06
10	ω_1 =	16.62889	rad/s		0.06	0.065777	-0.048674691	-0.04785	-0.00129374	-2.45378E-06
11	f_1 =	2.64657	Hz		0.07	0.063504	-0.046993152	-0.04191	-0.00174257	-1.76849E-05
12	ζ_1 =	0.0033			0.08	0.061549	-0.045545991	-0.03506	-0.00212741	-3.70919E-05
13	m_1^* =	1.005157			0.09	0.060357	-0.044664359	-0.02781	-0.00244172	-5.99979E-05
14					0.1	0.060173	-0.044528165	-0.02054	-0.00268345	-8.56843E-05
15	Newmark Coefficients				0.11	0.060825	-0.045010552	-0.01333	-0.00285281	-0.000113426
16	α =	0.5			0.12	0.061601	-0.045584633	-0.00586	-0.00294877	-0.000142496
17	β =	0.166667			0.13	0.061857	-0.045773878	0.002153	-0.00296729	-0.000172143
18					0.14	0.061563	-0.045556597	0.0105	-0.00290403	-0.000201569
19					0.15	0.06112	-0.045228799	0.018659	-0.00275823	-0.000229948
20	Initial Conditions				0.16	0.060828	-0.045012432	0.026185	-0.00253401	-0.000256472
21	d_o =	0	m		0.17	0.060709	-0.044924986	0.032855	-0.00223881	-0.000280392
22	v_o =	0	m		0.18	0.060653	-0.044883375	0.038567	-0.0018817	-0.000301042
23					0.19	0.060541	-0.044800393	0.043254	-0.00147259	-0.000317853
24					0.2	0.060319	-0.044636076	0.046826	-0.00102219	-0.000330356
25					0.21	0.060005	-0.04440355	0.049174	-0.00054219	-0.000338198
26					0.22	0.059668	-0.044154408	0.050184	-4.5399E-05	-0.000341144
27					0.23	0.059424	-0.043973866	0.049743	0.00045424	-0.000339096
28					0.24	0.059387	-0.043946302	0.047782	0.00094186	-0.000332099
29					0.25	0.059559	-0.044073342	0.044356	0.00140255	-0.000320349
30					0.26	0.059832	-0.04427556	0.039637	0.00182252	-0.000304184
31					0.27	0.060157	-0.044516398	0.033796	0.00218968	-0.000284074

Figure 8: Creating a Copy of 1st Mode Worksheet

Step 6 (continued)

Then check the box for "Create a copy" and click on "OK" button (Fig. 9)

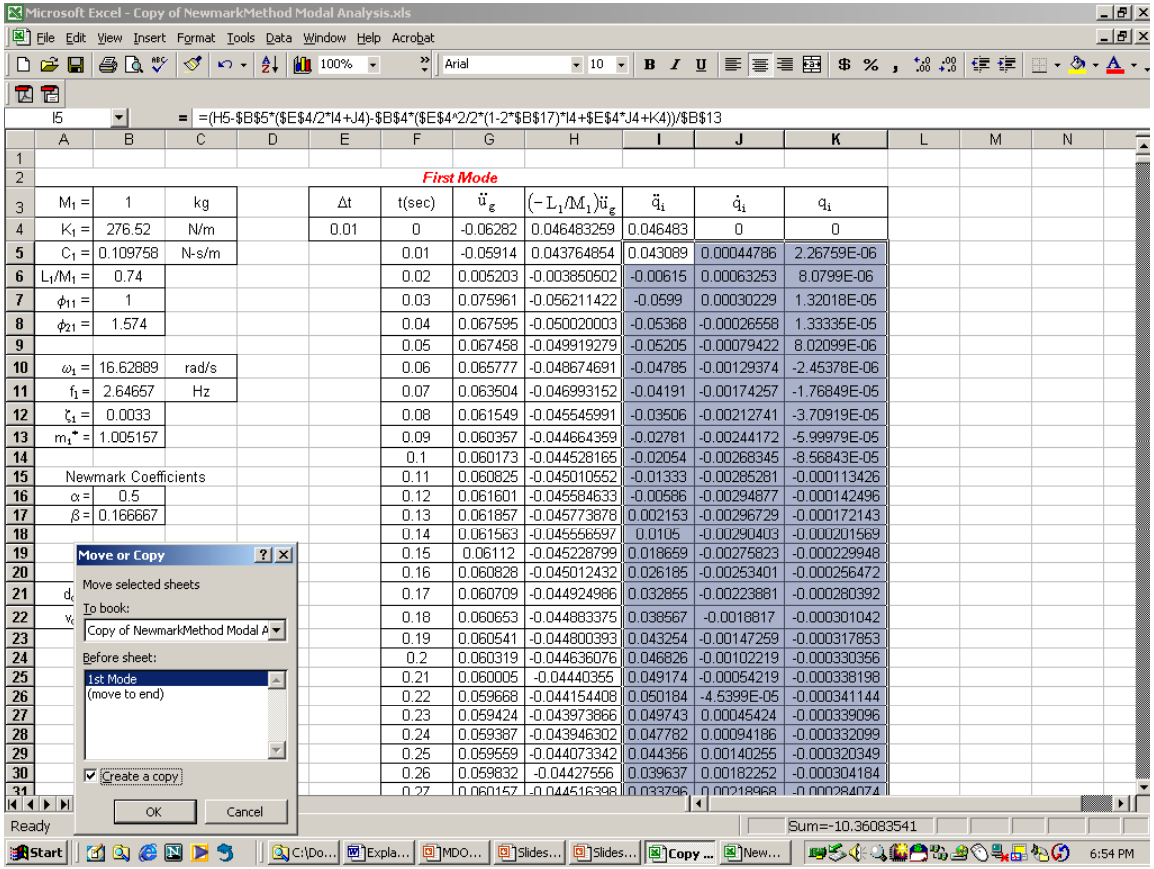


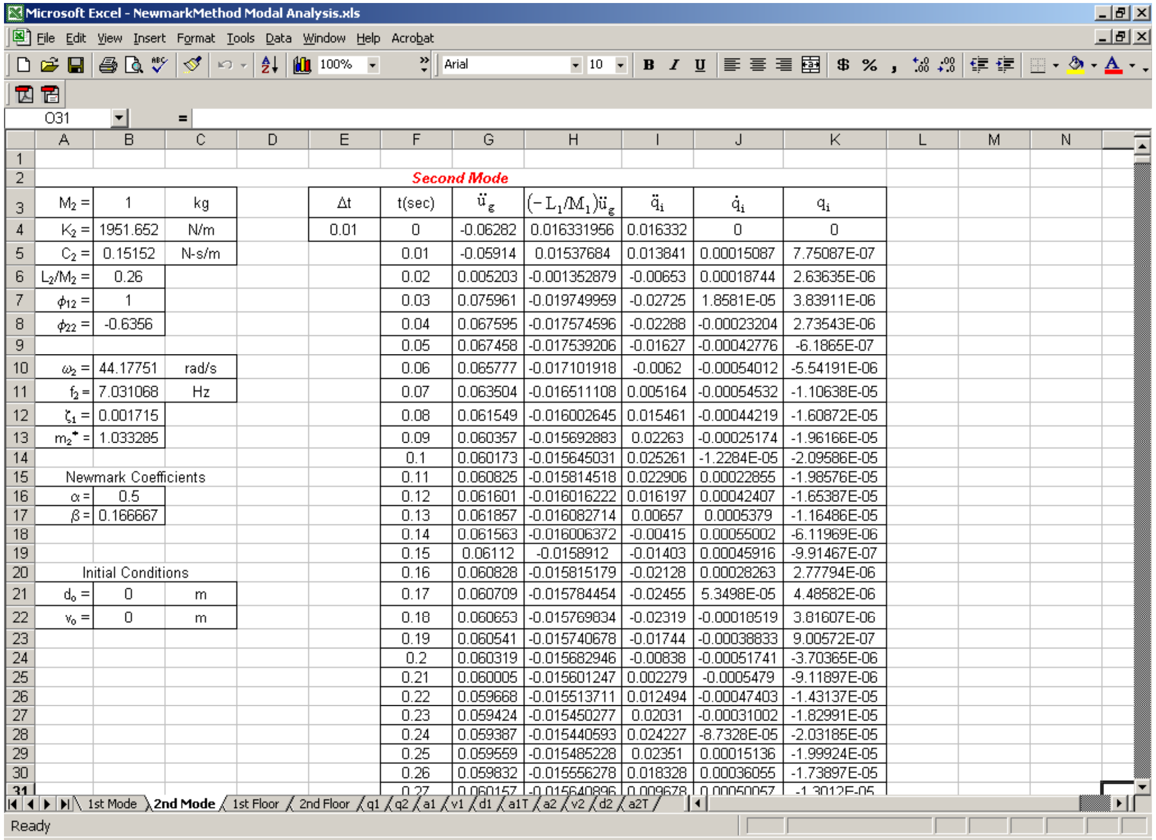
Figure 9: Creating a Copy of 1st Mode Worksheet

Step 6 (continued)

Rename this worksheet by right clicking on the "1st Mode (2)" tab and selecting "Rename". Rename this worksheet "2nd Mode" (Fig. 10)

Enter the appropriate values for M_2, K_2, C_2, $\frac{L_2}{M_2}$, ϕ_2, d_o, and v_o (Fig. 10).

Step 6 (continued)



Microsoft Excel - NewmarkMethod Modal Analysis.xls

Second Mode

M_2 =	1	kg
K_2 =	1951.652	N/m
C_2 =	0.15152	N-s/m
L_2/M_2 =	0.26	
ϕ_{12} =	1	
ϕ_{22} =	-0.6356	
ω_2 =	44.17751	rad/s
f_2 =	7.031068	Hz
ζ_1 =	0.001715	
m_2^* =	1.033285	
Newmark Coefficients		
α =	0.5	
β =	0.166667	
Initial Conditions		
d_o =	0	m
v_o =	0	m

Δt	t(sec)	$\ddot{u}_g$	$(-L_1/M_1)\ddot{u}_g$	$\ddot{q}_i$	$\dot{q}_i$	q_i
0.01	0	-0.06282	0.016331956	0.016332	0	0
	0.01	-0.05914	0.01537684	0.013841	0.00015087	7.75087E-07
	0.02	0.005203	-0.001352879	-0.00653	0.00018744	2.63635E-06
	0.03	0.075961	-0.019749959	-0.02725	1.8581E-05	3.83911E-06
	0.04	0.067595	-0.017574596	-0.02288	-0.00023204	2.73543E-06
	0.05	0.067458	-0.017539206	-0.01627	-0.00042776	-6.1865E-07
	0.06	0.065777	-0.017101918	-0.0062	-0.00054012	-5.54191E-06
	0.07	0.063504	-0.016511108	0.005164	-0.00054532	-1.10638E-05
	0.08	0.061549	-0.016002645	0.015461	-0.00044219	-1.60872E-05
	0.09	0.060357	-0.015692883	0.02263	-0.00025174	-1.96166E-05
	0.1	0.060173	-0.015645031	0.025261	-1.2284E-05	-2.09586E-05
	0.11	0.060825	-0.015814518	0.022906	0.00022855	-1.98576E-05
	0.12	0.061601	-0.016016222	0.016197	0.00042407	-1.65387E-05
	0.13	0.061857	-0.016082714	0.00657	0.0005379	-1.16486E-05
	0.14	0.061563	-0.016006372	-0.00415	0.00055002	-6.11969E-06
	0.15	0.06112	-0.0158912	-0.01403	0.00045916	-9.91467E-07
	0.16	0.060828	-0.015815179	-0.02128	0.00028263	2.77794E-06
	0.17	0.060709	-0.015784454	-0.02455	5.3498E-05	4.48582E-06
	0.18	0.060653	-0.015769834	-0.02319	-0.00018519	3.81607E-06
	0.19	0.060541	-0.015740678	-0.01744	-0.00038833	9.00572E-07
	0.2	0.060319	-0.015682946	-0.00838	-0.00051741	-3.70365E-06
	0.21	0.060005	-0.015601247	0.002279	-0.0005479	-9.11897E-06
	0.22	0.059668	-0.015513711	0.012494	-0.00047403	-1.43137E-05
	0.23	0.059424	-0.015450277	0.02031	-0.00031002	-1.82991E-05
	0.24	0.059387	-0.015440593	0.024227	-8.7328E-05	-2.03185E-05
	0.25	0.059559	-0.015485228	0.02351	0.00015136	-1.99924E-05
	0.26	0.059832	-0.015556278	0.018328	0.00036055	-1.73897E-05
	0.27	0.060157	-0.015640896	0.009678	0.00050057	-1.3012E-05

1st Mode | 2nd Mode | 1st Floor | 2nd Floor | q1 | q2 | a1 | v1 | d1 | a1T | a2 | v2 | d2 | a2T

Figure 10: Worksheet for Second Mode

Step 7 – Repeat Step 6 for Additional Modes

Step 8 – Determine the Response at Each of the Floors

Determine the Response of the first floor using the equations:

$$\mathbf{u} = \mathbf{\Phi q}$$

$$\dot{\mathbf{u}} = \mathbf{\Phi}\dot{\mathbf{q}}$$

$$\ddot{\mathbf{u}} = \mathbf{\Phi}\ddot{\mathbf{q}}$$

Step 8 (continued)

For example for a 2DOF structure, the first floor response is (Fig. 11)

$$u_1 = \phi_{11} q_1 + \phi_{12} q_2 \qquad \text{(Equation 11)}$$

$$\dot{u}_1 = \phi_{11} \dot{q}_1 + \phi_{12} \dot{q}_2 \qquad \text{(Equation 12)}$$

$$\ddot{u}_1 = \phi_{11} \ddot{q}_1 + \phi_{12} \ddot{q}_2 \qquad \text{(Equation 13)}$$

Step 8 (continued)

and the second floor response is (Fig. 12)

$$u_2 = \phi_{21} q_1 + \phi_{22} q_2 \qquad \text{(Equation 14)}$$

$$\dot{u}_2 = \phi_{21} \dot{q}_1 + \phi_{22} \dot{q}_2 \qquad \text{(Equation 15)}$$

$$\ddot{u}_2 = \phi_{21} \ddot{q}_1 + \phi_{22} \ddot{q}_2 \qquad \text{(Equation 16)}$$

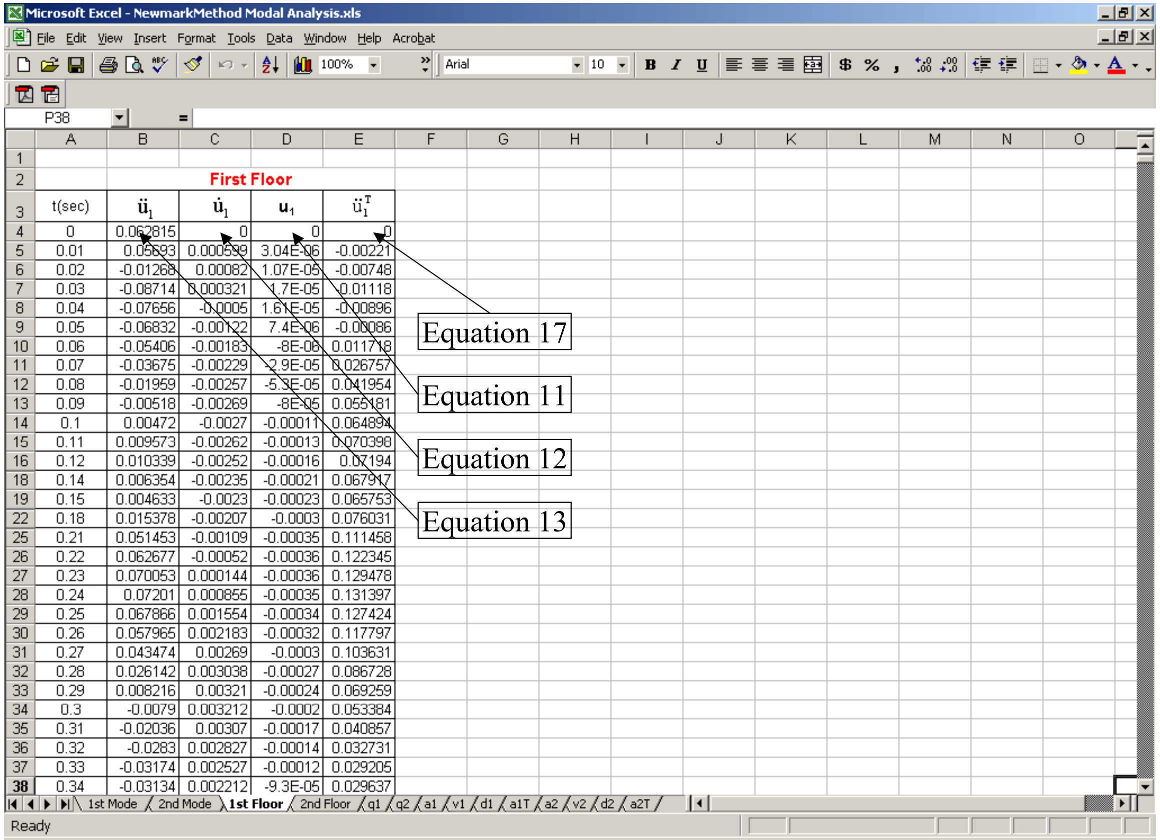
The first floor absolute acceleration is

$$\ddot{u}_1^T = \ddot{u}_1 + \ddot{u}_g \qquad \text{(Equation 17)}$$

The second floor absolute acceleration is

$$\ddot{u}_2^T = \ddot{u}_2 + \ddot{u}_g \qquad \text{(Equation 18)}$$

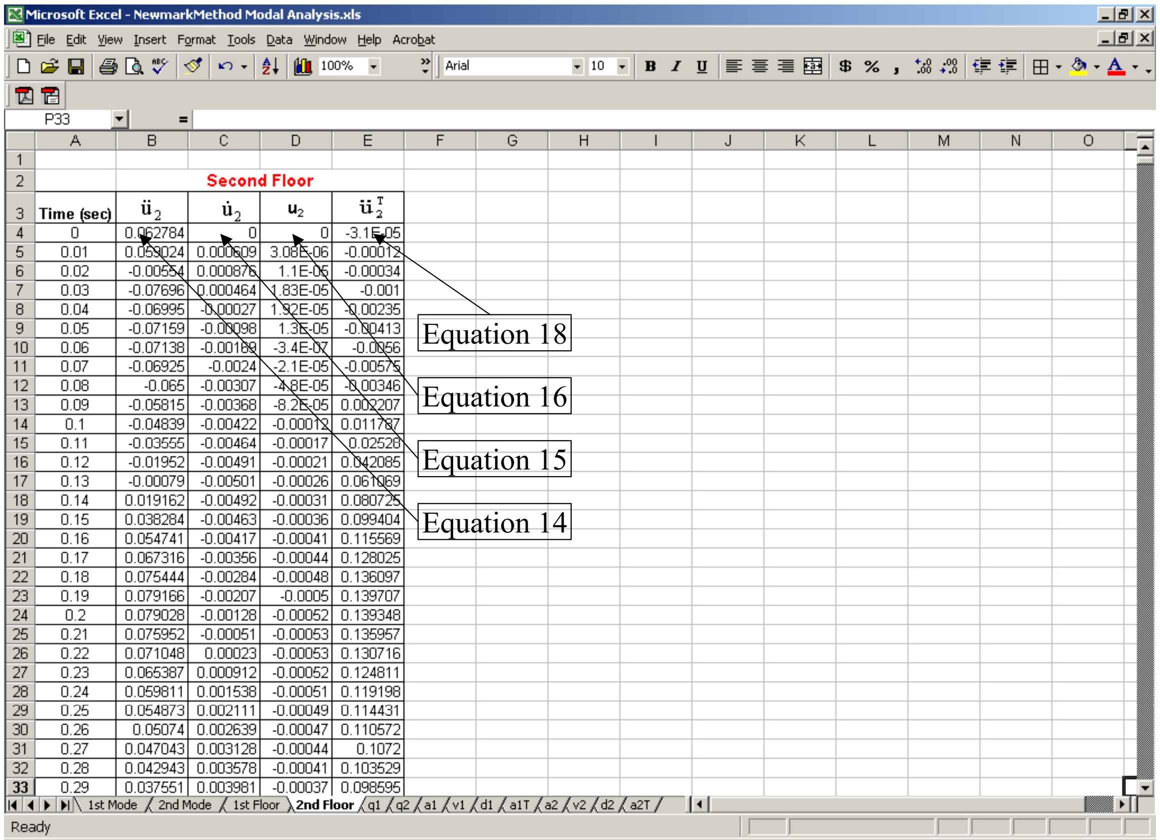
Step 8 (continued)



t(sec)	$\ddot{u}_1$	$\dot{u}_1$	u_1	$\ddot{u}_1^T$
0	0.062815	0	0	0
0.01	0.05693	0.000599	3.04E-06	-0.00221
0.02	-0.01268	0.00082	1.07E-05	-0.00748
0.03	-0.08714	0.000321	1.7E-05	-0.01118
0.04	-0.07656	-0.0005	1.61E-05	-0.00896
0.05	-0.06832	-0.00122	7.4E-06	-0.00086
0.06	-0.05406	-0.00183	-8E-06	0.011718
0.07	-0.03675	-0.00229	-2.9E-05	0.026757
0.08	-0.01959	-0.00257	-5.3E-05	0.041954
0.09	-0.00518	-0.00269	-8E-05	0.055181
0.1	0.00472	-0.0027	-0.00011	0.064894
0.11	0.009573	-0.00262	-0.00013	0.070398
0.12	0.010339	-0.00252	-0.00016	0.07194
0.14	0.006354	-0.00235	-0.00021	0.067917
0.15	0.004633	-0.0023	-0.00023	0.065753
0.18	0.015378	-0.00207	-0.0003	0.076031
0.21	0.051453	-0.00109	-0.00035	0.111458
0.22	0.062677	-0.00052	-0.00036	0.122345
0.23	0.070053	0.000144	-0.00036	0.129478
0.24	0.07201	0.000855	-0.00035	0.131397
0.25	0.067866	0.001554	-0.00034	0.127424
0.26	0.057965	0.002183	-0.00032	0.117797
0.27	0.043474	0.00269	-0.0003	0.103631
0.28	0.026142	0.003038	-0.00027	0.086728
0.29	0.008216	0.00321	-0.00024	0.069259
0.3	-0.0079	0.003212	-0.0002	0.053384
0.31	-0.02036	0.00307	-0.00017	0.040857
0.32	-0.0283	0.002827	-0.00014	0.032731
0.33	-0.03174	0.002527	-0.00012	0.029205
0.34	-0.03134	0.002212	-9.3E-05	0.029637

Figure 11: First Floor Response

Step 8 (continued)



Time (sec)	$\ddot{u}_2$	$\dot{u}_2$	u_2	$\ddot{u}_2^T$
0	0.062784	0	0	-3.1E-05
0.01	0.059024	0.000609	3.08E-06	-0.00012
0.02	-0.00554	0.000876	1.1E-05	-0.00034
0.03	-0.07696	0.000464	1.83E-05	-0.001
0.04	-0.06995	-0.00027	1.92E-05	-0.00235
0.05	-0.07159	-0.00098	1.3E-05	-0.00413
0.06	-0.07138	-0.00169	-3.4E-07	-0.0056
0.07	-0.06925	-0.0024	-2.1E-05	-0.00575
0.08	-0.065	-0.00307	-4.8E-05	-0.00346
0.09	-0.05815	-0.00368	-8.2E-05	0.002207
0.1	-0.04839	-0.00422	-0.00012	0.011787
0.11	-0.03555	-0.00464	-0.00017	0.02528
0.12	-0.01952	-0.00491	-0.00021	0.042085
0.13	-0.00079	-0.00501	-0.00026	0.061069
0.14	0.019162	-0.00492	-0.00031	0.080725
0.15	0.038284	-0.00463	-0.00036	0.099404
0.16	0.054741	-0.00417	-0.00041	0.115569
0.17	0.067316	-0.00356	-0.00044	0.128025
0.18	0.075444	-0.00284	-0.00048	0.136097
0.19	0.079166	-0.00207	-0.0005	0.139707
0.2	0.079028	-0.00128	-0.00052	0.139348
0.21	0.075952	-0.00051	-0.00053	0.135957
0.22	0.071048	0.00023	-0.00053	0.130716
0.23	0.065387	0.000912	-0.00052	0.124811
0.24	0.059811	0.001538	-0.00051	0.119198
0.25	0.054873	0.002111	-0.00049	0.114431
0.26	0.05074	0.002639	-0.00047	0.110572
0.27	0.047043	0.003128	-0.00044	0.1072
0.28	0.042943	0.003578	-0.00041	0.103529
0.29	0.037551	0.003981	-0.00037	0.098595

Figure 12: Second Floor Response